IIFC Conference Proceedings to be Indexed

Jian-Fei Chen and Kent A. Harries

The IIFC is pleased to announce that Elsevier is now indexing post-2012 IIFC conference proceedings in the Scopus and Compendex indices.

Elsevier approached IIFC past president Prof. Larry Bank with the opportunity to include CICE conferences in the Scopus and Compendex indices. It was felt that this was a great benefit to our membership and to authors. Prof. Bank and current president Prof. Jian-Fei Chen requested that APFIS conferences also be included. We are pleased to announce that Elsevier agreed and IIFC formally granted permission for indexing.

With assistance from Professors Riadh Al-Mahaidi, Yu Tao and Raafat El-Hacha, proceedings of CICE2012, CICE2014 and APFIS2013 have been passed over to Elsevier for indexing. All future proceedings will also be indexed. This is an excellent development, benefiting IIFC, authors and future researchers who will have greater access to IIFC proceedings. It is our experience that once Elsevier begins indexing a source, other indexing resources follow, further increasing the impact of the conference series and the outstanding work of all the authors.

This retroactive indexing is an indication of the high standards of IIFC conference proceedings, and demonstrates that the IIFC, as the world’s premier learned society for FRP composites in construction, is growing from strength to strength.
Long Term Bending Creep Behaviour of Thin-Walled CFRP Pretensioned High Strength Spun Concrete Poles Under Sustained Load

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Introduction and Aim

Driven by the need for more durable and sustainable concrete structures, careful selection, design, and optimization of both the concrete mixes and the reinforcing materials used is now commonplace within the precast concrete industry. An example of this is the use of precast CFRP pretensioned High Performance Spun Concrete (HPSC) members for lighting poles and electrical power line masts (Terrasi, 2013). The combination of CFRP and HPSC along with an appropriate interface between them (i.e. an epoxy-bonded sand-coating on the CFRP) allow minimisation of the weight of prestressed elements by reducing concrete cover and wall thickness while providing excellent serviceability (corrosion resistance, high stiffness and fatigue strength). However, the performance of these HPSC precast elements under long term bending creep loads is still not well known. When the authors started their research on the mechanical behaviour and production technology of slender CFRP prestressed spun concrete (Terrasi, 1998) the durability and creep resistance of such structures was an early and major concern, too (Terrasi et al., 2002). Creep test data were already available for the individual components such as the unidirectional CFRP pultrusion profiles or the epoxy and adhesives, but long term data for a slender pole as a structural element with its new material combination were still missing. Therefore, in 1996, the authors started a unique and still ongoing experiment with prestressed poles under permanent bending loads. The goal of the study was the proof of long term stability of the poles even under elevated loads with a setup able to expose all different kind of time dependent failure mechanisms (stress relaxation in the tendons, bond failure, stress corrosion, concrete creep). The chosen load levels were 36%, 50%, and 72% of the short term failure moment. The first failure of a pole after 16.5 years proved the plausibility of the applied load levels in order to show the limits given by the bond strength between CFRP-tendons and concrete.

Materials and Experimental Program

The structural elements in the current study were CFRP prestressed concrete cylinders. 3.0 mm diameter, pultruded, alumina sand-coated CFRP prestressing tendons were used. The carbon fiber used was T700S by Toray Japan with a volume fraction of 72% and the tendons had an epoxy polymer resin matrix with a glass transition temperature, \( T_g \), of 110 °C. The tendons’ average experimental tensile strength was 3375 MPa with an elastic modulus \( E_{11} \) of 180.7 GPa and an average ultimate strain of 1.87% (Terrasi, 1998). One should note that the carbon fibre T700S has a guaranteed ultimate tensile strain of 2.1%, which could not be reached in tensile testing due to premature anchorage failure. In order to increase bond to the concrete, the tendons’ surface was coated with an epoxy (type Scotch Weld 2216 by 3M, average layer thickness 0.25 mm) bonded layer of 0.5 mm diameter Al203 sand granules (with a compressive strength of 4000 MPa). Additionally, a novel kind of shear reinforcement was used for the poles consisting of a CFRP tape spiral with polyamide matrix of pitch 40 mm and cross-section 7 mm x 0.3 mm (Terrasi, 1998) (Fig. 1). The low density, excellent stress-corrosion resistance, and low creep and relaxation of CFRP are well known (Uomoto, 2001). The above properties make unidirectional CFRP tendons particularly suitable as prestressing reinforcements for concrete elements (Burgoyne, 1997).

Fig. 1 Cross-section of CFRP prestressed pole specimens and bending load arrangement.
The current study used a high performance, centrifugally cast concrete (HPSC) of strength class C115 (minimum 150 mm cube strength after 28 days of 115 MPa); this material is used for producing slender precast, prestressed poles and pylons in Switzerland (Terrasi, 2013). The HPSC is characterized by a precise grain size distribution of selected 0-6 mm quartz sand aggregates, with a cement content of 600 kg/m³ (CEM I 52,5 R). Silica fume (54 kg/m³ were added), and high performance superplasticizers play a key role in the mix. This particular HPSC mix design allows for optimum spinning of thin-walled cylinders at water/(cement+silica fume) ratios in the range of 0.31-0.32. Concrete compaction was carried out by centrifugation for 15 minutes with a maximum revolution speed of 800 rpm in a pretensioning-spinning mould (Terrasi, 1998). Prestress was released after 2 days and then the elements were demoulded. The pole specimens were then kept in a 20°C/90% R.H. chamber for 7 days and thereafter left to cure under indoor ambient conditions.

Five thin walled (25-27 mm total thickness) 2.3 m long cylindrical specimens (poles) with an outer diameter of 100 mm were centrifugally cast by precaster SACAC AG in Lenzburg, Switzerland (Fig. 1 and Table 1). These cross-section dimensions correspond to the smallest lighting pole cross-section produced by SACAC for the European market. The eight CFRP tendons were stressed to an initial prestress of 1600 MPa leading to a central HPSC prestress of 12.5 MPa after 28 days. The CFRP prestress wasn’t monitored after demoulding but the prestress losses were analytically estimated to be 16.8% after 28 days from centrifugation. This was calculated considering the experimentally determined E-modulus for the HPSC after 2 days from production (30 GPa), the exp. determined shrinkage strain of the concrete (0.0007 after 28 days) and the exp. determined creep coefficient (0.62) of the concrete under similar central compression at 20°C and 70% R.H. The tendons were evenly distributed on a circle with diameter 74 mm, had a cover of 11 mm and a tendon-to-tendon clear distance of 25 mm. Three DEMEC strain gauges were placed on the tensile and compressive edge in the center-span (base length 200 mm, accuracy ±0.002 mm) while midspan deflections δ was measured with an LVDT in the quasi-static bending tests and by a mechanical gauge in the bending creep tests (both with accuracy ±0.02 mm). Slippage of the top and bottom CFRP tendon (next to the compressive and tensile edge respectively) was monitored at the poles’ end surfaces with the aid of a sliding calliper (accuracy ±0.1 mm). Crack positions and crack widths were recorded during the tests with the aid of a crack microscope with accuracy ±0.02 mm, which was applied on the pole surface 20 mm under the horizontal axis of symmetry of the cylinder.

Poles 7 and 8 were tested in quasi static 4-point bending at age 28 days, with lever arms of 667 mm, a span of 2000 mm and 150 mm of overhang on each end (Fig. 1). In the main long term experiment Poles 12, 13 and 14 were tested from age 28 days onwards with the same span and load arrangement in outdoor 4-point bending creep tests (Fig. 1). In the outdoor bending creep experiments concrete blocks of known mass were hung in the 2 thirds-points of the pole specimens. Lead plates fixed thereon allowed the precise adjustment of the selected load level, taking into account the mass of the steel loading devices (Fig. 4). Pole 12 was loaded with a constant bending moment of 2.07 kNm, which is just below the short-term cracking

<table>
<thead>
<tr>
<th>Pole</th>
<th>CFRP prestress (MPa)</th>
<th>Test</th>
<th>Cracking moment (kNm)</th>
<th>Creep moment (kNm)</th>
<th>Failure moment (kNm)</th>
<th>Time to failure (h or y)</th>
<th>Midspan deflection at failure (mm)</th>
<th>Δε_{c,max} (%)</th>
<th>Δε_{CFRP,max} (%)</th>
<th>Failure mode</th>
</tr>
</thead>
<tbody>
<tr>
<td>7</td>
<td>1600</td>
<td>quasi static</td>
<td>2.58</td>
<td>n.a.</td>
<td>5.76</td>
<td>1 h</td>
<td>66.4</td>
<td>-0.00537</td>
<td>0.01055</td>
<td>HPSC crushing</td>
</tr>
<tr>
<td>8</td>
<td>1600</td>
<td>quasi static</td>
<td>2.41</td>
<td>n.a.</td>
<td>5.53</td>
<td>1 h</td>
<td>70.6</td>
<td>-0.00590</td>
<td>0.01133</td>
<td>HPSC crushing</td>
</tr>
<tr>
<td>12</td>
<td>1600</td>
<td>creep</td>
<td>2.07</td>
<td>2.07</td>
<td>none</td>
<td>running</td>
<td>n.a.</td>
<td>-0.00237</td>
<td>0.00155</td>
<td>none</td>
</tr>
<tr>
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<td>1600</td>
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<td>n.a.</td>
<td>2.80</td>
<td>none</td>
<td>running</td>
<td>n.a.</td>
<td>-0.00414</td>
<td>0.00392</td>
<td>none</td>
</tr>
<tr>
<td>14</td>
<td>1600</td>
<td>creep</td>
<td>n.a.</td>
<td>4.07</td>
<td>4.07</td>
<td>16.54 y</td>
<td>68.6*</td>
<td>-0.00684*</td>
<td>0.00927*</td>
<td>bond</td>
</tr>
</tbody>
</table>

* obtained 1.4 years before failure
moments measured in the flexural test of Pole 7 and Pole 8 (2.41 – 2.58 kNm, Table 1). The bending creep moment of Pole 13 (2.8 kNm) corresponded to the maximum short-term service moment of a 5-meter high lighting pole under wind load (SN 505 160, 1989) and was just above the short-term cracking moments. The highest loaded specimen Pole 14 was loaded (considering self-weight) with a bending moment of 72% of the average short-term failure moment determined in the flexural tests of Poles 7 and 8 (5.65 kNm, Table 1).

Results and Discussion

Table 1 shows the main test results of the two short-term bending tests to failure of the reference Poles 7 and 8, and of Poles 12, 13 and 14 tested in bending creep. Unfortunately the maximum creep deflection and maximum creep strain values for Pole 14 were lastly taken 1.4 years before failure. Furthermore, the maximum bending strain $\Delta \varepsilon_{\text{CFRP, max}}$ of the outermost CFRP tendon (near the tensile edge of the pole) is not measured directly, but can be calculated by the measured values of the maximum compression strain at the top edge of the pole $\Delta \varepsilon_{\text{c, max}}$ and the maximum tensile strain at the lower edge $\Delta \varepsilon_{\text{t, max}}$ (Fig. 1). These last two strains were both determined by measuring over the three DEMEC gauges with base length of 200 mm. In this case the assumption is that 'plane sections remain plane' during loading, which was experimentally proven in (Terrasi, 1998) to be a valid assumption.

![Fig. 2 Quasi-static bending moment vs. curvature behavior of CFRP prestressed HPSC Pole 7.](image)

The short term bending behavior was assessed in quasi static 4-point flexural tests of Poles 7 and 8. Both specimens showed an approximately bilinear load-deflection behavior with considerable deflection until failure by crushing of the HPSC compression zone. The crushing strains of the HPSC are remarkably high and reached an average value of 0.006 (see $\Delta \varepsilon_{\text{c, max}}$ in Table 1 on top of which the initial concrete strain due to prestress of 0.00036 has to be added). The high values $\Delta \varepsilon_{\text{CFRP, max}}$ of the outermost CFRP tendons (around 0.011, Table 1) show that the outermost tendon reaches considerable tensile stresses if one takes into account that the tendon strain resulting from prestress after 28 days is 0.073 (considering prestress losses (Terrasi, 1998)). Fig. 2 shows the experimental moment vs. curvature diagram for Pole 7, with the curvature in the zone of pure bending derived from the strain measurements on the top and bottom flanges during flexure. In the lower part of the diagram the pole remained uncracked till the bending stresses overcame the centric prestress and the tensile strength of the HPSC (which was determined to be 10.9±1.3 MPa in standard three-point bending tests of unreinforced spun cylinders of age 28 days). The kink in the moment-curvature curve was caused by the formation of the first bending cracks in the tensile zone of the central pole span. With the load increasing, the number of cracks in the central span increased, and the cracked zone developed from the central span to the shear spans. Therefore, the curvature (and deflection) increased considerably. The experimental curvatures in the cracked state and the failure moment at HPSC crushing could be predicted well by analytically calculated values following the direct cross-sectional analysis described in (Terrasi, 1998).

The main aim of this work was studying the long term behavior of the structural elements. The time-dependent load and deformation behavior of the CFRP prestressed HPSC Poles 12, 13 and 14 was studied in three 4-point bending creep tests performed outdoors between December 1996 and July 2013. Fig. 3 shows the creep deflection at midspan vs. time for the three poles loaded at the 3 different load levels of Table 1. The midspan deflections of the specimens show a rapid increase during the first 6 months after loading. This is followed by a rather slow increase of deflections, which increase slightly during the warmer summer months, while they keep stable during the colder part of the year. A possible explanation for this would be the fact that creep of the HPSC under sustained bending compression and HPSC shrinkage are accelerated by higher temperatures.
Fig. 3 Bending creep tests of Poles 12, 13 and 14: Creep deflection at midspan vs. time.

The two less loaded Poles 12 (loaded with the maximum long-term design bending moment for a pole after (SN 505 160, 1989)) and 13 did not fail during the long term test; both experiments are still running and will be monitored in the years to come. The highest loaded specimen Pole 14 sustained a bending moment of 72% of the short-term bending failure moment for 16.5 years before failing July 4th 2013 due to bond failure of the bottom tendon, which led to local crushing of the HSPC in the ninth crack section located 0.76 m from the left pole end (Fig. 4). The debonding of the Al203 sand coating from the CFRP tendon surface could be clearly observed at the tendon end and by carefully removing the HPSC cover at the tensile edge of the failed cross-section of Pole 14 (Fig. 5, left part of the tendon). Note that this highest loaded CFRP tendon of Pole 14 was strained to 0.0136 (2457 MPa) at the beginning of the creep test and that 1.4 years before failure its strain increased to 0.01628 (2942 MPa). This increased tendon stress was calculated from the measured values of $\Delta \varepsilon_{c,\text{max}}$ at the top edge and $\Delta \varepsilon_{t,\text{max}}$ at the lower edge of the pole and shows the stress redistribution over the section due to bending creep and crack growth. It is hypothized that this increased bottom tendon strain and the coating’s bond creep led to the CFRP anchorage failure by debonding of the Al203 sand coating from the CFRP tendon surface after 16.5 years.

Slippage monitoring for the top and bottom CFRP tendon ends of Poles 12 and 13 (next to the poles’ compressive and tensile edge, respectively) showed a limited draw-in of the tendons between 0.8 – 1.6 mm (top tendons) and 0.8 - 1.1 mm (bottom tendons) in 16.5 years. It is hypothized that one part of these slippages is an artifact caused by erosion of the epoxy matrix of the tendon free ends by UV radiation. This was confirmed by optical microscope analysis of the free tendon ends, which showed a clear erosion after 16.5 years. Therefore the effective draw-in of the highest tension-loaded (bottom) tendons is assumed to be considerably less than the measured 0.8 - 1.1 mm. This limited tendon slippage is a proof for the long-term durability of the bond between HPSC and CFRP in mid-European climatic conditions, achieved through Al203 sand coating of the CFRP surface: Bond creep at the HPSC / CFRP interface is thus very limited over 16.5 years. This result confirms the above hypothesis that creep and shrinkage of the HPSC are controlling the long-term deformations of the pole specimens.

Fig. 4 Failure of Pole 14 due to slippage of the lowest CFRP tendon in the tensile zone on the left end of the pole, which led to local crushing of the high strength spun concrete.

Fig. 5 Detail of tensile edge of failed cross-section of Pole 14 after removing HPSC cover/

Conclusions
The following conclusions can be drawn on the basis of the test results briefly described in this paper:
The long term bending behavior of the CFRP prestressed HPSC pole studied is satisfactory under realistic service moments, which follows from the low creep deflections after 16.5 years for the lowest loaded Pole 12 in the test series presented. Long-term curvatures and deflections stabilize after 6 months of sustained loading. Furthermore, the pole specimens loaded with 36% and 50% of the short term failure moments showed crack patterns which were stable over time, and minimal slippage of the tendons with respect to the pole's end faces. The latter proves the successful and durable anchorage of the Al203 sand coated CFRP prestressing tendons of this study in thin-walled precast concrete members under sustained long-term service loads.

Pole 14, which was loaded with twice the maximum long term service moment, failed after 16.5 years due to bond failure of the highest loaded CFRP tendon.

Acknowledgements

The authors would like to thank SACAC Schleuderbetonwerk AG. This research was also funded by EMPA and by the Swiss Commission for Technology and Innovation, to which we are very grateful.

References


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**FiberCore Europe installs two new bridges**

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On 12 January, the wooden bridge deck of the bascule **Friesebrug** in Alkmaar, Netherlands, was replaced with a span incorporating an FRP deck featuring InfraCore®Inside technology (right). Situated at the entrance to the city of Alkmaar, the Friese Bridge is used by all types of traffic. The use of the FRP reduced the weight of the bridge leaf providing many advantages when opening and closing the bridge. In addition, the FRP deck provides protection for the underlying steel construction against water and de-icing salt reducing maintenance demands. Gebr. Griekspoor BV were the principal contractors for this project.

The movable part of the new **Spiering Bridge**, featuring a light-weight bridge leaf constructed using InfraCore®Inside technology, was installed over the river Vecht 14 January, 2015. The Spiering bridge is an essential link for local traffic in the Weesp and Muiden (Netherlands) area.

The bridge leaf is made of FRP attached to a steel girder on either side (green in the photograph below) The finish of leaf is a wear layer; an additional tarmac layer is not required.

This bridge leaf is one of a number of movable bridges that have been constructed using lighter and maintenance-friendly FRP materials. This trend in movable bridges represents a successful combination of FRP and steel. The properties of these materials complement each other, allowing ease of bonding. This has already been illustrated on by the 142 metre long traffic viaduct near Lunetten (Netherlands) and by a number of lock gates.

The fibre-reinforced polymer bridge leaf was manufactured by FiberCore Europe. The steel construction for this project was executed by Jansen Venneboer (Wijhe, Netherlands). The principal contractors for the project were K. Dekker B.V. / Ooms Construction B.V.

The FRP leaf was connected to the steel at the Rotterdam-based production facility of FiberCore Europe. The assembled bridge leaf was shipped by water from Rotterdam to Nigtevecht on the Amsterdam-Rijn canal, it was forwarded to Muiden in the morning of 14 January, 2015.

**Happy 50th Birthday, Kevlar!**

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DuPont is celebrating the 50th anniversary of a widely popular advanced material – Poly-paraphenylene terephthalamide, better known as Kevlar®. First discovered by Stephanie Kwolek while researching for a new lightweight strong fiber to use for tires, the material has been used in countless applications over the last five decades from personal protective gear to paint to dog toys (and the editor’s favourite: ice hockey socks that protect one’s calves from errant skate blades.)
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An experimental study on repair of steel bridge piles using GFRP concrete-filled jacket
A. Kaya
MS Thesis (2014), University of Houston
Advisors: B. Gencturk and M. Dawood

Many bridges and structures in the United States that are supported on steel piles exhibit inadequate strength due to increasing load demand and aging due to corrosion. The combination of increased load demand and reduction of capacity due to corrosion-induced section loss can lead to unexpected buckling of the piles. Several techniques are available to repair these structures to meet the increasing demand and enhance their safety. This thesis investigates the effectiveness of a glass fiber reinforced polymer (GFRP)-based system for rapid repair of buckled steel piles. The system consists of a GFRP tube, which is formed on-site and subsequently filled with an expansive concrete. Thirteen buckled steel H-piles with varying degrees of section loss to simulate corrosion were repaired and tested to failure under axial loading. The research results show that the repair system can restore the capacity of the piles comparable to the undamaged conditions.

Calibration of φ factors for CFRP prestressed bridge girders
F. Forouzannia
MS Thesis (2014), University of Houston
Advisors: B. Gencturk and M. Dawood

Calibration of the flexural resistance factors in the American Association of State Highway and Transportation Official’s (AASHTO) Load and Resistance Factor Design (LRFD) format is performed for bridge girders prestressed with Carbon Fiber-Reinforced Polymers (CFRP). The underlying principle of the LRFD design is to achieve a uniform probability of failure (target reliability) for all possible design scenarios, which is achieved through resistance and load factors. Calibration of the resistance factors requires an extensive design space to be applicable to different design scenarios. For this purpose, 12 design cases with various span lengths, girder positions, girder spacing, roadway widths, and failure modes were considered. The load and resistance model random variables and their statistics, flexural resistance model accuracy, and the results of Monte Carlo simulation through which resistance factors were derived for different target reliabilities for interior and exterior girders failing in tension and interior girders failing in compression are presented.

Bond behavior between steel and high modulus CFRP plates at moderately elevated temperatures
M.U. Sahin
MS Thesis (2014), University of Houston
Advisor: M. Dawood

This thesis presents the findings of a research study that was conducted to assess the effect of moderately elevated temperatures, up to 50°C, on the bond behavior of steel beams strengthened with externally bonded carbon fiber reinforced polymer (CFRP) plates. In the first phase of the research, five steel-CFRP bonded double-lap shear coupons were tested at different temperatures to characterize the bond behavior. In the second phase of testing steel beams were strengthened with different configurations of high modulus CFRP plates and subjected to different combinations of applied load and ambient temperature. The temperature ranges considered were selected to represent typical environmental conditions experienced by many steel bridges and structures in different environments. The parameters that are considered in this study include the length of the CFRP plate and the combined effect of mechanical and thermal load.

New Publication

ACI SP-301 Modeling of FRP Strengthening Techniques in Concrete Infrastructure

This CD contains 8 papers that were presented at a session sponsored by Joint ACI-ASCE technical committee 447 at the ACI Fall Convention, October 2011 in Cincinnati, Ohio. The papers cover the modeling for strengthening for flexure, shear, torsion, and confinement of concrete. Where applicable, the papers cover comparisons of modeling results with experimental tests performed around the world. Document may be ordered at: http://www.concrete.org/store/productdetail.aspx?ItemID=SP301CD&Format=OPTICAL_DISK
ASCE Journal of Composites for Construction

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Seismic Behavior of RC Shear Walls Strengthened for In-Plane Bending Using Externally Bonded FRP Sheets
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Introduction of a Stress State Criterion to Predict Bond Strength between FRP and Concrete Substrate
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Shear Strength of FRP Reinforced Concrete Members with Stirrups
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Investigation of FRP Lap Splice Using Epoxy Containing Carbon Nanotubes
Eslam M. Soliman, Usama F. Kandil, and Mahmoud M. Reda Taha

Finite Element Modeling of Insulated FRP-Strengthened RC Beams Exposed to Fire
Jian-Guo Dai, Wan-Yang Gao, and J. G. Teng

Fatigue Behavior of Full-Scale Slab Bridge Strips with FRP Reinforcement
Martin Noël and Khaled Soudki

Seismic Strengthening of Masonry-Infilled RC Frames with TRM: Experimental Study
L. Koutas, S. N. Bousias, and T. C. Triantafillou

Torsional Moment Capacity and Failure Mode Mechanisms of Concrete Beams Reinforced with Carbon FRP Bars and Stirrups
Hamdy M. Mohamed, Omar Chaallal, and Brahim Benmokrane

CALL FOR PAPERS
International Journal of Polymer: Special Issue on Applications of Fiber Reinforced Polymer Composites
Lead Guest Editor: Togay Ozbakkaloglu, University of Adelaide
Guest Editors: Jian-Fei Chen, Queen's University Belfast; Scott T. Smith, Southern Cross University; and Jian-Guo Dai, Hong Kong Polytechnic University

Fueled by the need to surpass the limitations of conventional materials, recent years have seen a large increase in engineering applications of advanced fiber reinforced polymer (FRP) composite materials in many major industries. While there are numerous advantages offered by FRP composites in various engineering applications, there are still a number of technical and implementation issues that need to be resolved prior to broader acceptance of the application of FRP composites by some engineering communities such as civil construction. This special issue is aimed at disseminating the most recent advances and developments in this exciting field. We invite authors to submit original research and review articles dealing with cutting-edge issues on research and application of FRP composites.

Authors can submit manuscripts via the Manuscript Tracking System at http://mts.hindawi.com/submit/journals/ijps/frpc/

Manuscripts Due: Friday, 5 June 2015
IJPS is an Open Access Journal, publication may be subject to article processing charges.
Upcoming Conferences and Meetings

International Conference on Advances in Composite Materials and Structures, April 13-15, 2015, Istanbul, Turkey. sites.google.com/site/cacmsistanbul2015/


CAMX: Composites and Advanced Materials Expo, October 26-29, 2015, Dallas TX, USA. www.thecamx.org


7th International Conference on Advanced Composite Materials in Bridges and Structures, August 22-25, 2016 Vancouver, Canada.

CICE 2016 8th International Conference on FRP Composites in Civil Engineering December 2016, Hong Kong

CICE 2018 9th International Conference on FRP Composites in Civil Engineering July 2018, Paris

CICE 2014 Proceedings available on IIFC website SOON!

Proceedings of the following official IIFC conferences are archived on the IIFC website, www.iifc-hq.org:

CICE 2014, Vancouver, 20-22 August 2014
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CICE 2010, Beijing, China, 27-29 September 2010
APFIS 2009, Seoul, Korea, 9-11 December 2009
CICE 2008, Zurich, Switzerland, 22-24 July 2008
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By submitting this application you agree to share your contact information with fellow IIFC members and any conference/organization associated with IIFC.
**FRP International needs your input...**

As IIFC grows, we seek to expand the utility and reach of FRP International. The newsletter will continue to report the activities of IIFC and focus on IIFC-sponsored conferences and meetings. Nevertheless, we also solicit short articles of all kinds: research or research-in-progress reports and letters, case studies, field applications, book reviews or anything that might interest the IIFC membership. Articles will generally run about 1000 words and be well-illustrated. Submissions may be sent directly to the editor. Additionally, please utilize FRP International as a forum to announce items of interest to the membership. Announcements of upcoming conferences, innovative research or products and abstracts from newly-published PhD dissertations are particularly encouraged. All announcements are duplicated on the IIFC website (www.iifc-hq.org) and all issues of the FRP International are also available in the archive at this site.

FRP International is yours, the IIFC membership’s forum. The newsletter will only be as useful and interesting as you help to make it. So, again, please become an FRP International author.

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- L.P. Ye: Tsinghua University, China
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Key dates
Submission of Abstracts 30 November 2014
Abstract Acceptance Notification 15 December 2014
Submission of Full Lengths Papers 31 March 2015
Paper Acceptance Notification 10 June 2015
Submission of Final Revised Papers 15 July 2015
Early Registration 30 June 2015
SMAR 2015 Conference 7 – 9 September 2015
The official language of the conference is English

Registration Fee
Early Bird Registration until 15 June 2015 € 500.00 and € 250.00 for students
Standard Registration after 15 June 2015 € 600.00 and € 350.00 for students

Registration fee includes:
Conference proceedings hard cover including extended abstracts and CD with full papers, conference reception and conference dinner, 3-days catering, coffee breaks and lunches
Accompanying persons (none scientists) and visitors to exhibition € 200.00*
*Including: Conference reception, conference dinner, 3-days catering, coffee breaks and lunches

Accommodations see: www.smar2015.org

Scientific Tours
to be announced on Conference website: www.smar2015.org soon

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Scope
The International Conference on Smart Monitoring, Assessment and Rehabilitation of Civil Structures, SMAR 2015, will provide a forum for international scientists, engineers, enterprisers and infrastructure managers to present and discuss the state-of-the-practice and recent advances in testing and monitoring technology, in structural modeling and assessment methods, and in the application of advances materials for structural rehabilitation. The conference will provide a platform for exploring the potential of international cooperation.

SMAR 2015 is the third conference and will be sponsored jointly by the International Society for Structural Health Monitoring of Intelligent Infrastructure (ISHMII), the International Institute of FRP in Construction (IIFC), RILEM and Turkish Chamber of Civil Engineers (TMMOB).

Topics
Structural Health Monitoring
- Smart sensors
- Wireless sensor networks
- Implementation of structural monitoring
- Monitoring of off-shore structures and oil pipelines
- Monitoring of high rise buildings and bridges
- Advanced inspection and testing
- System identification and model updating

Performance and damage assessment
- Safety evaluation and reliability forecast

Damage control, repair and strengthening
- External strengthening using FRP composites
- Strengthening of concrete, timber and steel structures
- Strengthening of masonry and historic structures
- Confinement of concrete columns
- Near surface mounting reinforcement
- Seismic Retrofitting

Durability issues as related to harsh environments

Special session: Shape memory alloys in civil structures
- Fire protection systems

Practical applications and case studies
Visionary Concepts

Mirko Roš award for the best papers
- 1 paper in the field of “monitoring and assessment” and
- 1 paper in the field of “rehabilitation of civil structures”
Selected best papers will be proposed to be published in a Journal Special Issue dedicated to SMAR 2015 conference.

Conference Venue
City/Country: Antalya / Turkey
Date: 7 – 9 September 2015
Venue: Ramada Plaza Antalya
Gencilik Mahallesi Fevzi Çakmak Cadessi No 22
TR – 07100 Antalya
www.ramadaplazaantalya.com/

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Keynote Lecturers
Christoph Czaderski, Empa “Iron based shape memory alloys – a new material for prestressing of concrete structures”
Michael N. Fardis, University of Patras “Experience from the use of the European Standard EN1998-3:2005 on Seismic Assessment and Retrofitting of Buildings and prospects for the future”
Daniele Inaudi, SMARTEC “Point, quasi- and fully-distributed optical fiber sensing: what to use for SHM?”
Koichi Kusunoki, University of Tokyo “Automatic Damage Classification with SHM – We must know the condition of structures right after an earthquake”
Mehdi Saiid Saiidi, CATBI “Smart Materials for Accelerated Bridge Construction in High Seismic Zones”

Sponsors
Local and international companies and institutions providing sensors, monitoring systems, FRP composites and other materials and systems for structural rehabilitation.

Exhibition
Manufacturers and exhibitors in the field of smart monitoring, assessment and rehabilitation of Civil Structures are invited to reserve their stands at the conference SMAR 2015.
Join us in Brisbane in 2015

You are invited to attend the **Second International Conference on Performance-based and Lifecycle Structural Engineering (PLSE 2015)** to be held in Brisbane, Australia from 9th to 11th December 2015.

This upcoming event will be organised by the School of Civil Engineering, The University of Queensland together with the Research Institute for Sustainable Urban Development (RISUD), The Hong Kong Polytechnic University.

PLSE 2015 will provide an international forum for scientific exchanges in performance-based, and lifecycle structural engineering and related topics. Extended versions of selected papers will also be considered for publication in three prestigious international peer-reviewed scientific journals, *Advances in Structural Engineering, Engineering Structures*, and *Fire Safety Journal*. Keynote speakers include:

- Professor Mark A. Bradford
- Professor Gengdong Cheng
- Professor Bruce R. Ellingwood
- Professor Dan Frangopol
- Professor Yozo Fujino
- Professor Guo-qiang Li
- Professor Stephen A. Mahin
- Professor Robert E. Melchers
- Professor David A. Nethercot
- Dr Man-Chung Tang

Visit the Conference website for more information and to join our mailing list:

[www.plse2015.org](http://www.plse2015.org)

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**Key Dates**

- **Abstract submission deadline**: 25 February 2015
- **Abstract author notification & Call for full papers**: 1 April 2015
- **Full paper submission deadline**: 1 June 2015
- **Full paper author notification**: 3 August 2015
- **Early bird & author registration deadline**: 16 September 2015
FRPRCS-12 / APFIS-2015
The 12th International Symposium on Fiber Reinforced Polymers for Reinforced Concrete Structures
& The 5th Asia-Pacific Conference on Fiber Reinforced Polymers in Structures
Joint Conference

(First Announcement and Call for Abstracts)
14-16 December 2015
Nanjing, China

Organized by
International Institute for Urban Systems Engineering
& National and Local Unified Engineering Research Center for Basalt Fiber Production and Application Technology
& School of Civil Engineering
Southeast University

APFIS is the Official IIFC Conference for the Asia-Pacific Region
FRPRCS-12 / APFIS-2015
14-16 Dec. 2015, Nanjing, China

**Topics**
(Including, but not limited to)
- Characterization of FRP
- New FRP materials/systems/techniques
- Durability, long-term performance of FRP
- Bond behavior
- Confinement
- Seismic strengthening
- Strengthening of concrete, metallic, timber and masonry structures
- Performance under extreme loading
- Advanced numerical models and simulations
- Structures reinforced or prestressed with FRP systems
- Hybrid structures
- All FRP structures
- Structural health monitoring and intelligent sensing
- Codes, standards, guidelines
- Field applications, case studies
- High performance, longevity, and sustainability of structures with FRP

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- X.L. Zhao, Australia
- K. Zilch, Germany

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(Southeast University)

**Key dates**
- Abstract submission—15th Jan. 2015
- Approval of abstracts—15th Feb. 2015
- Submission of papers—15th Apr. 2015
- Review outcome of papers—15th Jun. 2015
- Submission of finalized papers—15th Aug. 2015

**Registration fees**
- Early Bird (before 15th July)/Normal Registration: Full 600/700 USD
- Student 300/350 USD
- Accompanying Person: 250 USD

The registration fee includes IIIFC membership, lunches, proceedings and banquet dinner.

Contact email: frprcs-apfis2015@seu.edu.cn